Process Improvement on Domestic Wastewater Treatment Stabilization Ponds by Using Mathematical Optimization Approach

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Abstract In this paper, a mathematical optimization approach is used to improve the pollutant degradation process on domestic wastewater treatment stabilization ponds. The optimized process in this research is the pumping process that was modelled as a mathematical optimization problem. By using data that was collected in Sewon wastewater treatment in Yogyakarta, Indonesia, the pumping process can be optimized and the optimal decision was determined that can be applied to operate this wastewater treatment pond.

Keywords Domestic wastewater treatment; optimization; pollutant transport

Mathematics Subject Classification 90C20, 90C90, 62P12

1 Introduction

Wastewater treatment plants have been used in many countries to reduce the pollutant in the wastewater. Commonly, this treatment plants consisting of several ponds that were used to reduce the pollutant by using the natural processes by taking advantage from the bacteria, algae and zooplankton to reduce the organic pollutant [1]. The main ponds in the wastewater treatment plant are stabilization pond or facultative pond and maturation pond. The main purpose of this wastewater stabilization pond is to store and then to process the wastewater with some desired retention time [2]. Some mathematical models have been developed as an approach to analyze, observe and optimize the performance of the wastewater stabilization pond. For example, a linear optimization mathematical model was developed to optimize the performance of facultative pond [3]. Other approaches were performed, for example by simulating the effectiveness and purification efficiencies of the BOD [4], optimization model by minimizing the cost construction [5] by using natural adsorbent [6] and their extensions like sewage treatment in extreme area [7], analysis of nitrogen in waste stabilization ponds in the removal mechanism point of view [8] and maturation pond optimization [9].
Advanced researches were also conducted in order to maintain the ponds better, for example in [10] the advanced statistical analysis was applied to optimize the stabilization pond, in [11] an optimization model was governed to reduce the feed in the distillation column of the pond, in [12] it was considered the variables influence the phosphorus content in a waste stabilization pond, in [13] it was considered an integrated model of hydraulic, physico-chemical and micro-biological processes, in [14] it was considered the combination model of oxygen electrode and biological model, in [15] the Indicator Organism and Organic Matter was optimized by a mathematical optimization model proposed in the research. The more advanced researches were conducted to take advantage of the material residue in wastewater pond as to produce Soil-Cement Bricks [16] and reuse of the water from the pond [17].

In this paper, we develop a model to optimize the pollutant reducing process on facultative stabilization pond in Sewon Bantul wastewater treatment plant. The model is developed to determine the optimal volume of the wastewater that will be processed on the facultative stabilization pond and to determine how long the storing time of the wastewater so that the efficiency value of the pond will near as close as possible to the efficiency reference value.

2 Methodology

The methodology used in this research is explained as follows. First, we employ the assumptions hold by the model approach which are (1) all units of wastewater volume in a day is the average of the observed data (2) we calculate the efficiency percentage of a pond by value of BOD degradation rate times the storing time day divided by itself plus one (3) the standard of the wastewater quality is different from one region/country to the others, then in this research we apply the quality standard that decided by the local government (Yogyakarta Province). In the next step, we identify and define all parameters/variables in which influence the pond in the model. We decide to include the wastewater load in inlet, inflow rate in the inlet and stabilization pond, wastewater volume prior to the pond, wastewater volume processed in the pond, concentration of BOD in the pond, sum of all wastewater volume in four observed ponds, efficiency value at each pond, and quality standard value of the wastewater.

Formulation of the mathematical optimization model is the next crucial step. In order to gain the best overall performance of the wastewater treatment, we decide to maximize the total wastewater volume in all ponds, and furthermore we decide to minimize the squared difference between the treatment efficiency value at each pond to a reference value decided by the decision maker in the wastewater plant. This means that the decision maker or manager wants to bring the treatment efficiency value to some value referred by himself. By combining these two objectives we formulate the objective function of the model as minimizing the total wastewater load minus total squared difference of efficiency value. The constraints that should be hold are that the quality standard requirement and wastewater volume processed in the ponds. The observed data from Sewon Bantul Yogyakarta wastewater treatment ponds, then we substituted it into the formulated model. Finally, by using quadratic optimization solver in LINGO 17.0 software, we solve the optimization problem to obtain the optimal decisions which are discovering how much of the wastewater volume that should be processed in the pond and how much the corresponding treatment efficiency value that will be achieved.
3 Mathematical Model

The formulated model is covering the facultative ponds only and it does not include the maturation pond that illustrated in Figure 1.

![Diagram of Sewon Bantul Wastewater Treatment Plant]

Figure 1: Sewon Bantul Wastewater Treatment Plant

The processing of the wastewater is started in inlet and divided into four facultative stabilization ponds. The model was formulated as a quadratic optimization problem where the variables and the parameters are:

- $L_0$ - Initial wastewater load volume in inlet (kg/day)
- $Q_0$ - Wastewater inflow rate in the inlet (m$^3$/day)
- $Q_{p}^e$ - Wastewater inflow rate in facultative pond $p$ (m$^3$/day)
- $L_p$ - Wastewater load volume prior to pond $p$ (kg/day)
- $L_{pe}$ - Wastewater volume processed in pond $p$ (kg/day)
- $C_p$ - Concentration of Biological Oxygen Demand at pond $p$ (mg/L)
- $L$ - Wastewater load for all ponds (kg/day)
- $E_p$ - Efficiency value of the treatment at pond $p$ (in percentage)
- $E_{p}^r$ - Efficiency reference value of the treatment at pond $p$ (in percentage) decided by director or DM
- $BM$ - Quality standards of wastewater decided by the local government
The objective function that will be optimized has two components. The first objective is the total volume of the processed wastewater in all ponds that should be maximized. The second objective is the total squared value of the difference between treatment efficiency value and the reference efficiency value for all ponds that should be minimized. This reference efficiency value will be determined by the decision maker which means that the decision maker wants to bring the wastewater process efficiency value to the given reference as close as possible. Hence, the model was formulated as follows:

\[
\max Z = \sum_{p=1}^{P} L_p^e - \sum_{p=1}^{P} (E_p - E_r^p)^2
\]  

subject to:

\[E_p C_p \leq BM, \forall p \in P; \]  

\[L_p^e \leq L_p, \forall p \in P; \]  

The value of \( L_p^e \) is formulated as \( L_p^e = \frac{(Q_p^e C_p)}{1000} \) or \( C_p = \frac{(1000L_p)}{Q_p^e} \) which means that the volume of the input wastewater on these facultative ponds is the flow rate of the wastewater including the organic matter i.e. Biochemical Oxygen Demand (BOD). The efficiency of each facultative pond is calculated by using the following formula:

\[E_p = \frac{k \cdot t}{1 + k \cdot t}\]  

where \( k \) is the BOD degradation rate and \( t \) is the storing time (in day) of the wastewater in the pond. Hence, the mathematical model can be rewritten as follows:

\[
\max Z = \sum_{p=1}^{P} L_p^e - \sum_{p=1}^{P} (E_p - E_r^p)^2
\]  

subject to:

\[E_p \left(\frac{1000L_p}{Q_p^e}\right) \leq BM, \forall p = 1, 2, 3, 4; \]  

4 Results and Discussions

The observation data to evaluate model were taken in Sewon WWTP located in Bantul district, Yogyakarta Province, Indonesia. Data value for some parameters were used in our previous works [3]. The inflow rate of the wastewater \( (Q_0) \) is assumed to be the average of the inflow rate for one year which is 11,238.38 m\(^3\)/day and the wastewater processing time in the facultative
ponds is 4.83 day on average. The wastewater load volume in facultative pond I and II are a half of the inflow load which is \( L_0 = 4,799.6 \text{ kg/day} \) and the BOD for \( L_1 \) and \( L_2 \) is the half which is 2,399.8 kg/day. The wastewater quality standards that we used is 50 mg/L as the local governments’ policy [18]. The optimization problem (5) was solved in LINGO 17.0 by using daily used personal computer with Windows 10 x64 of operating system, Dual Core 3.0 GHz of processor and 4 GB of memory. Based on the data collected in the inlet, the BOD concentration is 68.3 mg/L – 177.2 mg/L in outlet is 13 mg/L-18,8 mg/L and the used BOD degradation coefficient is 1.1 [3].

Table 1: Optimization Result

<table>
<thead>
<tr>
<th>Pond (p)</th>
<th>( L_p )</th>
<th>( E_p )</th>
<th>( E_p^* )</th>
<th>( Q_p^* )</th>
<th>( L_p )</th>
<th>( k(p) )</th>
<th>( t(p) )</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>1199.5</td>
<td>0.280846</td>
<td>0.5</td>
<td>13475</td>
<td>2399</td>
<td>1.1</td>
<td>0.355018</td>
</tr>
<tr>
<td>2</td>
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<td>0.280846</td>
<td>0.5</td>
<td>13475</td>
<td>2399</td>
<td>1.1</td>
<td>0.355021</td>
</tr>
<tr>
<td>3</td>
<td>1199.5</td>
<td>0.561692</td>
<td>0.9</td>
<td>13475</td>
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<td>1.1</td>
<td>1.165002</td>
</tr>
<tr>
<td>4</td>
<td>1199.5</td>
<td>0.561692</td>
<td>0.9</td>
<td>13475</td>
<td>1199.5</td>
<td>1.1</td>
<td>1.165002</td>
</tr>
</tbody>
</table>

From the optimization results given in Table 1, it can be concluded that the optimal wastewater volume in each pond should be 1199.5 kg per day where the treatment efficiency of the pond I and II is 28% and the treatment efficiency value of pond III and IV is 0.56%. The corresponding storing time in pond I and II is 0.35 day and in pond III and IV is 1.16 day.

5 Conclusions

In this paper, a mathematical model in a quadratic optimization form was considered which was used to optimize the wastewater processing in Sewon wastewater treatment plant. The results was shown that the optimal wastewater volume of each pond should be 1199.5 kg/day and the storing time at pond I and II is 0.35 day which had 0.28 of the treatment efficiency and the storing time at pond III and IV is 1.16 day which had 0.56 of the treatment efficiency.

In the future works, we will develop the model so that it can be used to optimize the wastewater processing including the maturation pond. Furthermore, there will be more data collected to verify and determine the parameter value via parameter estimation of the model so that the accuracy of the model will be more clarified.

References


